

Abstract

High-resolution reflection seismic methods are an established non-destructive tool for engineering tasks. In the near surface, shear wave reflection seismic measurements usually offer a higher spatial resolution in the same effective signal frequency spectrum than P wave data, but data quality varies more strongly.

To discuss the causes of these differences, we investigated a P wave and a SH wave reflection seismic profile measured at the same location on Föhr island, and applied reflection seismic processing to the field data as well as finite difference modelling of the seismic wavefield (SOFI FD-code). The simulations calculated were adapted to the acquisition field geometry, comprising 2 m receiver distance and 4 m shot distance along the 1.5 km long P wave and 800 m long SH wave profiles. A Ricker-Wavelet and the use of absorbing frames were first order model parameters. The petrophysical parameters to populate the structural models down to 400 m depth are taken from borehole data, VSP measurements and cross-plot relations.

The first simulation of the P wave wavefield was based on a simplified hydrogeological model of the survey location containing six lithostratigraphic units. Single shot data were compared and seismic sections created. Major features like direct wave, refracted waves and reflections are imaged, but the reflectors describing a prominent till layer at ca. 80 m depth was missing. Therefore, the P wave input model was refined and 16 units assigned. These define a laterally more variable velocity model ($v_p = 1600\text{--}2300\text{ m s}^{-1}$) leading to a much better reproduction of the field data. The SH wave model was adapted accordingly but only led to minor correlation with the field data and produced a higher signal-to-noise ratio. Therefore, we suggest to consider for future simulations additional features like intrinsic damping, thin layering, or a near surface weathering layer. These may lead to a better understanding of key parameters determining the data quality of near-surface seismic measurements.

SED

6, 2169–2213, 2014

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



1 Introduction

Near surface geophysical methods constitute a non-destructive means to investigate the shallow subsurface. Especially engineering tasks, for instance geo-hazard assessment or groundwater prospecting, profit from structural and parametrical methods (e.g. Miller, 2013; Kirsch, 2008). In some cases, results of geophysical prospecting are compiled into 3-D models and can act as input for, e.g., groundwater flow modelling.

Among other geophysical methods high resolution reflection seismics constitutes a valuable tool to extract structural information. Reflection seismics using compressional waves (P waves) is an established method and often produces accurate results for near surface tasks (e.g. Steeples and Miller, 1990; Rumpel et al., 2009; Jørgensen et al., 2012). In many cases, however, a higher spatial resolution is desired than P waves offer. This is one important reason why shear waves have become popular. Since shear waves have a lower velocity than P waves, they offer shorter wavelengths, i.e., higher spatial resolution, within the same frequency band as P waves. Also, shear wave velocity information yields an additional parameter than the P wave velocity information alone and offers more detailed studies of elastic properties, like for instance Poisson's ratio. The combination of P and S wave velocities can help characterize lithology or pore fluid (e.g. Sheriff and Geldart, 1995). For shallow application, recent developments have been successful (e.g. Inazaki, 2004; Pugin et al., 2009a, b; Polom et al., 2010, 2013; Krawczyk et al., 2013). However, we experience that shear wave reflection seismics more strongly varies in quality compared with compressional wave seismics.

Seismic modelling is an important tool to evaluate seismic field data, yet not often applied for near surface tasks. Most studies applying wavefield modelling concentrate on deep reservoirs and crustal structures (Robertsson et al., 2007). A comprehensive approach is reported by Bellefleur et al. (2012), we use as well in this study. One current method to create synthetic seismograms of complex structures is finite difference (FD) modelling (Alterman and Karal, 1968). The advantage of the FD method is the ability to

SED

6, 2169–2213, 2014

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



ally accepts absorption models for P and S waves. The SH -version does not require compressional wave parameters.

Another important feature of the code is its ability to run simulations in parallel threads on multi-processor computers to save time. For such parallel simulation, the model can be split up into subgrids that the different CPUs called processing elements (PE) calculate independently. For every time step each PE updates the wave field in its subgrid and parameter exchange needs to be carried out with the grid points of neighbouring subgrids. This exchange can slow down the calculation that much that the use of all CPUs and a respective number of subgrids not necessarily delivers the fastest result. Here, the minimum computational time for one test model was achieved with 48 processors and a corresponding number of subgrids.

3 Test site Föhr

The test site is the North Sea island of Föhr (Fig. 1) that was investigated as a pilot area in the Interreg project CLIWAT (Harbo et al., 2011), co-financed by the European Union. The aim of CLIWAT was to analyse the influence of climate change to ground-water systems, and one major outcome was a hydrogeological model to forecast the groundwater evolution (Burschil et al., 2012a).

3.1 Geological evolution

The investigation area is located on the German North Sea island of Föhr, which is part of the UNESCO world heritage Wadden Sea (Fig. 1). In general, the deeper sub-surface was sedimented as part of the Northern German Basin, while the landscape of Föhr was formed during the glacial and post glacial epoch (Scheer, 2012). The older sediments were deposited under marine conditions since Cretaceous age until the youngest Tertiary. For that reason, marine clay (mica clay) is found up to Miocene age. Sedimentation changed during Pliocene and sandy material (kaolin sand) was de-

SED

6, 2169–2213, 2014

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



**FD modelling to
evaluate seismic field
data**

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



posited until a climate change marked the beginning of the Pleistocene age. Glaciations from the Baltic–Scandinavian area reworked the shallow underground by alternating processes of glacial advance followed by erosion and sedimentation. In the region of Föhr push moraines as well as a system of tunnel valleys were formed and refilled with glacial deposits (Scheer, 2012). The great outwash plains were increasingly flooded during Holocene so that tidal mud deposits were accumulated and formed large marshland areas. Finally, heavy floods in historical times eroded large parts and formed the present North Frisian Wadden Sea.

3.2 Geophysical and geological framework

In the project CLIWAT we accomplished a multidisciplinary geophysical acquisition (Burschil et al., 2012a). Between 2009 and 2011, we acquired seven reflection seismic profiles with P waves (in total 8 km) and three profiles with SH waves (in total 2.4 km). Five Vertical Seismic Profiles (VSP) were recorded with a 3C borehole geophone and the small electro-dynamical vibrator system ELVIS (Polom et al., 2011) with excitation in vertical and horizontal-transversal direction relative to the borehole location (Fig. 1). Maximum depths of five VSPs were in the range of 39–102 m, depending on the borehole.

Additionally, in 2008 the island was mapped with the airborne electromagnetic system SkyTEM (Sørensen and Auken, 2004). The result of the P wave seismic survey was used as a-priori information for the electromagnetic data inversion (Burschil et al., 2012b). The information transfer between the different geophysical methods improved the electric resistivity model of the island. Borehole logging data were evaluated statistically regarding electric resistivity as well as seismic velocity. This allowed a petro-physical classification of sand, till and clay (Burschil et al., 2012a). These lithological units form structures as push-moraines and buried valleys, which are consistent with the geological evolution of the region.

3.3 Hydrogeological model

The hydrogeological 3-D model for Föhr represents a simplification of the geological and geophysical information and only contains hydraulically relevant strata, i.e. aquifers and aquitards. With respect to the different geophysical methods applied (Burschil et al., 2012a), no lateral variations or internal structures were added to lithological units.

As a first approach for FD modelling we extracted a cross section along a seismic profile and assigned lithological information (Fig. 2). The cross section shows different units of sand and till above a bed of clay. The prominent till layer is interrupted in the middle of the cross section. Near the surface, thin lenses of till and silt are embedded in sand.

4 Reflection seismic field data

4.1 Seismic acquisition

Seismic equipment varied with surveys due to different wave types used for exploration. For two *P* wave surveys we used the hydraulic vibrator systems of the Leibniz Institute for Applied Geophysics (LIAG), the MHV2.7 in 2009 and the new HVP-30 in 2010. We used a linear sweep in the range of 30–240 Hz with 10 s duration. The source excited on a paved street in the western part of the profile and on grassland in the eastern part. The receivers were vertical geophones (20 Hz), planted every 2 m in the green strip next to the line of the source locations, with a maximum offset of 360 m to enhance the near surface resolution and fold. We used a combination of split-spread/roll along geometry (Fig. 3a) with a source spacing of two-times receiver spacing (4 m). With this geometry we had acquired high quality data before. We operated up to 268 active channels with Geometrics Geode seismographs. This resulted in a fold between 47

SED

6, 2169–2213, 2014

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



semblance calculations. We therefore included geological a-priori knowledge to reduce the uncertainty, so that the resulting interval velocity distribution better corresponds to the reflectors (Burschil et al., 2012a). At the end of the processing sequence we achieved depth-converted, time-migrated sections (Fig. 6).

The shear wave data contain surface wave interferences related to the specific shear wave reflection move out. This is the reason why we cannot simply mute the surface wave noise and purely focus on reflection signals, as we could for P waves. To enhance the reflection signals we applied several techniques, e.g. fk-filtering, spectral whitening, and deconvolution. Finally, automatic gain control (AGC, 300 ms) and fk-filtering with low-cut of velocities below 350 m s^{-1} provided the best results (Table 1). This filter also removes a wide range of the chevron pattern that is present in the lower part of the seismograms around the shot location, depicted in Fig. 5. In contrast to Polom (1997) who investigated a chevron pattern that originated in ghost sweeps, we cannot identify a comparable increase of the frequency with time in the data. Here, the chevron pattern is rather mono-frequent and no ghost sweeps can be detected within the pattern.

Velocity analysis was very difficult because reflections can only be identified sporadically and can hardly be traced through to neighbouring shots. This restrains velocity analysis in CMP-sorted data. After normal move out-correction and common midpoint-stacking we converted the section to depth with a velocity function derived from VSP measurements. The result is rather mono-frequent, but the till layer as well as deeper reflectors can be identified (Fig. 7). However, the quality of this shear wave survey is less compared to P wave seismic results.

5 Synthetic data from finite difference modelling

To understand data quality differences between shear wave measurements and compressional wave measurements on the island of Föhr, seismic wavefield modelling is introduced here for further data analysis. We chose the 2-D P/SV -version of SOFI for P wave simulations and the 2-D SH -version of SOFI for SH wave modelling.

6 Discussion

In reflection seismic surveys normally only one type of wave is utilized, for instance P waves, shear waves or converted waves. The use of more than one wave type or even the integration of multi-component technology (Hardage et al., 2011) is rare but growing. This is even more pronounced for near surface applications. Recently, a few studies reported the combination of shallow P wave and shear wave seismics, comparable to the field work presented in this article (e.g. Pugin et al., 2004, 2009b; Malehmir et al., 2013; Sauvin et al., 2014). Even 9C reflection seismics was tested in near surface exploration (Pugin et al., 2009a). All of these studies successfully recorded high resolution data of mainly good quality. The signal-to-noise ratio of the P wave data is similar to the P wave data presented in this study, except in (Pugin et al., 2009a). In contrast, shear wave data of these studies are of good quality that constitute in prominent, coherent reflections. Equipment and acquisition geometry slightly vary among the reported surveys, but the combination of vibrator and land streamer is the favourite choice for shear wave seismic. General data processing reported in some of the studies is similar to the processing we finally applied (Table 1). Sauvin et al. (2014) reported the application of elevation statics for only one of their shear wave profiles. None of the authors reported refraction statics for shear waves. In some cases, differences in shear wave processing are related to deconvolution and spectral whitening, which was applied by Pugin et al. (2009b) and Sauvin et al. (2014). Here, deconvolution and spectral whitening did not provide success.

So far, we have not been able to reproduce the shear wave field data with seismic modelling. A similar observation is reported by Bellefleur et al. (2012). They calculated synthetic P wave data of four different input models from the surface to the reservoir and compared these data with field data. Their data excellently correlate with data from a VSP, but they consider the correlation with a surface 3-D reflection seismic dataset as rather poor. They explain the poor surface data quality with the appearance of surface waves and a higher amount of scattering at inhomogeneities. Surface waves are not

SED

6, 2169–2213, 2014

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



Geldart, 1995). For *S* wave sources this mass increases horizontal friction of the base plate and thus prevents sliding of the shear wave vibrator unit on the ground. We use the rule of thumb that the gravitational hold down force of the vibrator mass on loose ground should at least be twice the peak shear force. Experience shows that under favourable conditions, the peak force of a vibrator with a rubber base plate on paved roads can reach 70 % of the hold down force without sliding. The *P* wave hydraulic vibrator systems MHV2.7/HVP-30 have masses of 2.6 t/4 t and a maximum peak forces of 27.6 kN/30 kN. Therefore, these vibrator systems should not be affected by bouncing. The shear wave vibrator system MHV-4S has a mass of 4 t and a maximum peak force of 30 kN. During the survey, we used a peak force of 17 kN on a paved road. Therefore, the gravitational hold down force of nearly 40 kN should have been sufficient.

Receiver coupling with the ground is another factor. Carefully planted geophones typically offer proper receiver ground coupling for *P* wave surveys (Krohn, 1984). The *P* wave field data we acquired support this expectation. For the shear wave survey we used the LIAG land streamer on which geophones are directly fixed to aluminium plates that have a gravitational three-point contact to the ground. This system has proven to receive good shear wave signals from the subsurface before (Polom et al., 2010; Krawczyk et al., 2013; Malehmir et al., 2013). On one *SH* wave profile on gravel Sauvin et al. (2014) used the same streamer but on grassland they planted the geophones in the ground. They do not report any difference in data quality. Pugin et al. (2004, 2009b, a) use a different land streamer system that works successfully as well. In the field, we took care that geophone coupling is good. Every time a new streamer position was reached, a noise test was carried out and noisy geophones were coupled to the ground by hand. We therefore expect coupling to be sufficient.

Surface waves are an important degradation factor for shear wave reflection surveys. Here, the reflection hyperbola often interferes with surface wave (Love wave) signals; cf. surface waves in Fig. 4 and reflection hyperbolas in Fig. 5. This factor has complicated the application of shear waves in the past and still limits the application of this method. However, the observation that a high velocity layer at the surface suppresses

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



the generation of surface waves in shear wave exploration (e.g. Inazaki, 2004) was an important step to overcome this limitation in many urban applications and even in rural environments, if paved or consolidated roads are present. Even though surface waves are often exited during *SH* wave surveys on unconsolidated surfaces, *SH* wave reflection seismic surveys have been successful in these conditions (e.g. Malehmir et al., 2013; Sauvin et al., 2014). Polom et al. (2013) identify partly dispersive Love waves that show a similar signature as the chevron pattern depicted in Fig. 5. In their case and in our case, these waves seem to be linked to the shot point location. Even neighbouring shot points show strong variations in this respect. For instance, in Fig. 5 we show shot gathers with shot points locations of FFID's 1126 and 1127 that are just 4 m apart. To cancel surface waves with a linear move out or mild dispersive character *fk*-filtering is often successfully used (e.g. Polom et al., 2013; Sauvin et al., 2014). We successfully applied an *fk*-filter as well (Table 2) but this did not improve the coherency of reflectors in the final depth section (Fig. 7).

In very complex structures, scattering is an important factor. It is closely related to energy loss through multiples, in case tuning layers or strong lateral impedance contrasts are present. In our case, at least in the upper 150 m a mixture of complex structures and thin layering can be expected (Fig. 2, Scheer, 2012). Compressional waves and shear waves could be influenced by this kind of energy loss in a different manner. In the same frequency band the wavelengths of *P* and *S* waves differ, depending on sub-surface velocity. If the sizes of subsurface structures are in the order of the wavelength of the main frequency present in the source signal a strong energy loss for the signals can be expected. This could be an important reason for the degradation of our shear wave data. In other cases, the shear wave data could be less degraded compared with the *P* wave data. The latter case is present in Pugin et al. (2009a). They relate the degradation to static problems, only occurring in the *P* wave measurements. Malehmir et al. (2013) report a case where this factor does not seem to negatively affect the data quality.

6.2.3 *P* wave depth section

Poststack migrated *P* wave sections of synthetic data show less noise and less amplitude variability while the corresponding field data show natural levels of different signals and thus contain more information about small-scale and internal structures (Figs. 6, 10, and 14). However, the poststack migrated section, derived from the hydrogeological model, does only reflect some of the main features imaged in the field data section. Important differences compared with the field data are the discontinuous reflector of the till surface at about at 80 m depth in the synthetic section (Fig. 10), and missing of a strong reflector at about 260 m depth as well as a number of dipping reflectors between about 50 and 180 m depth (Fig. 10).

The depth section of the modified input model better reflects the field data features (Fig. 14). The till top reflector between 50 m and 80 m depth appears continuous and two deep reflectors show as well. Some of the dipping reflectors in this modelled section add details that similarly appear in the field data.

The field data section (Fig. 6) can be divided into two parts: excitation on paved street and on grassland. Within the field data seismic section we detected a lack of resolution in the eastern part (about 900–1400 m) that is not present in the modelled data. However, we did not implement the structural complexity of unconsolidated grassland as a near surface weathering layer, i.e. large velocity contrasts, inhomogeneities, and intrinsic damping, in the model so far.

In general, the reflection signatures in the field data spatially vary more strongly than in the synthetic data (Figs. 6, 10, and 14). Sources for these observations in the field data can be intrinsic damping, scattering attenuation including fine layering, and inhomogeneous lithological units. All of these features were not included in the models (Figs. 8 and 11). In a natural environment, complex structures or fine layers can be sources of multiples and wave conversion that subsequently can pose similar challenges to data processing like random noise signals do. Since the model consisted of comparatively large units this kind of noise was not simulated.

SED

6, 2169–2213, 2014

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



In summary, this first and simplified input model is not able to reproduce major features in the seismic field data, but the modified model does reproduce these features.

6.2.4 *SH* wave stacked sections

5 The *SH* wave stacked sections of synthetic data (Fig. 16) and field data (Fig. 7) differ more than the *P* wave sections. While no clear interpretation can be carried out for the stacked field data, the stacked section from the modelled dataset reproduces well the input model. For example reflector segments occur at 90 m depth, which correspond to the interface of the sub-horizontal layer at 90 m (top till layer) and at 250 m depth
10 corresponding to the first deep reflector in the *P* wave seismic section. Nonetheless, the reflection signatures in the field data are of course much noisier than in the synthetic data.

7 Summary and outlook

15 Shear wave field data recorded on the island of Föhr showed less quality compared with their compressional wave counterparts. To comprehend the reasons for this quality difference we used seismic wavefield modelling within simple models of the subsurface, using the seismic field geometry. We chose finite difference modelling to try to reproduce the field data because of its ability to simulate the entire wavefield and to allow arbitrary input models. We come to the following conclusions:

- 20 1. After subsurface model optimization we were able to simulate *P* waves that show clear first order similarities compared with the *P* wave field data.
2. Simplified FD modelling does not explain the small signal-to-noise ratio of the shear wave field data.
- 25 3. We can simulate the chevron pattern that is present in the field data by clipping of uncorrelated channels near the source location.

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



For future analyses we therefore suggest to consider additional complexity in the subsurface model that will presumably be able to explain the different quality of compressional and shear wave field data. The most important additional factors are intrinsic damping, thin-layers within the modelled units, a complex near-surface weathering layer structure, and heterogeneous material within the layers. While 2-D calculations gain faster results and allow testing the effect of different features, the full complexity of field acquisition may be understood using 3-D simulations in the future.

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FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

◀

▶

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



- Fichtner, A., Bleibinhaus, F., and Capdeville, Y.: Full Seismic Waveform Modelling and Inversion, Springer, 2011.
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FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



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FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

⏪

⏩

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



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FD modelling to evaluate seismic field data

T. Burschil et al.

Table 1. Overview of processing of *P* wave and *SH* wave seismic field data including normal move out (NMO) correction, dip move out (DMO) correction, and common midpoint (CMP) stacking.

Processing step	<i>P</i> wave application	<i>SH</i> wave application
Data load	SEG2-data load to ProMAX	SEG2-data load to ProMAX
Geometry installation	1 m bin interval	0.5 m bin interval
Vibroseis correlation	applied in the field	using individual sweep
Vertical stacking of records	applied in the field	2 fold
Quality control	kill bad traces	kill bad traces
Refraction statics	calculated from first breaks; $v_{\text{replace}} = 1600 \text{ m s}^{-1}$	not applied
Amplitude scaling	Automatic gain control (300 ms length)	Automatic gain control (200 ms length)
Fan Filtering	not applied	Low cut 350 m s^{-1}
Deconvolution	Zero-phase spiking deconvolution (80 ms length)	not applied
Time-variant filter	Bandpass filter (36–220 Hz)	Bandpass filter (30–160 Hz)
Trace muting	Remove of noise cone	not applied
Interactive velocity analysis	100 m node spacing, iteratively	100 m node spacing, iteratively
Residual statics correction	correlation auto statics	not applied
NMO correction	RMS velocity function, 30 % stretch mute	RMS velocity function, 300 % stretch mute
DMO preparation	8 m DMO bin spacing, 24 bins	not applied
DMO correction	average single velocity function	not applied
Trace muting	top mute	not applied
CMP stacking		
Amplitude scaling	Automatic gain control (300 ms operator length)	not applied
Time-variant filter	Bandpass filter (passage window: 30–220 Hz)	not applied
Finite-Difference migration	smoothed velocity function	not applied
Time-to-depth conversion	Single velocity function from VSP	Single velocity function from VSP

Title Page

[Abstract](#) [Introduction](#)
[Conclusions](#) [References](#)
[Tables](#) [Figures](#)

⏪ ⏩
◀ ▶

[Back](#) [Close](#)

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



FD modelling to evaluate seismic field data

T. Burschil et al.

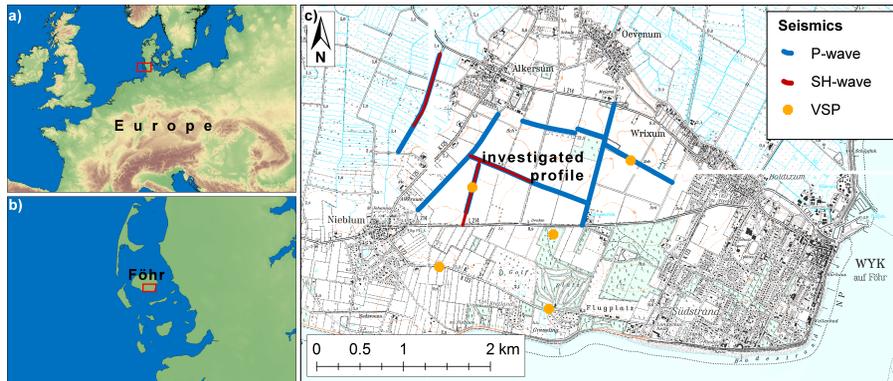


Figure 1. Overview maps with location of the island of Föhr (a, b). Detail map of the seismic locations on the island of Föhr (c). The profile discussed in this paper is labelled.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



**FD modelling to
evaluate seismic field
data**

T. Burschil et al.

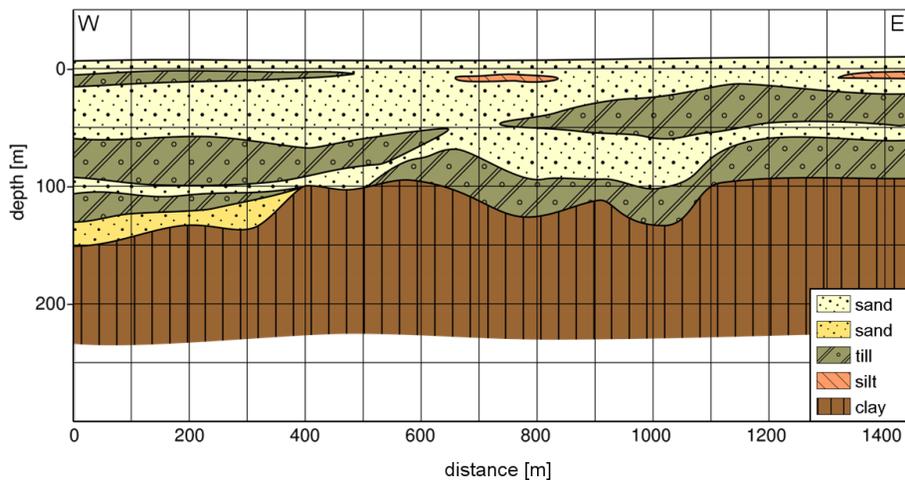


Figure 2. Cross section of the hydrogeological model with assigned lithologies, compiled from airborne electromagnetic, borehole information, and P wave seismics (after Burschil et al., 2012a). The location of the profile is shown in Fig. 1c.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures

◀

▶

◀

▶

Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



FD modelling to evaluate seismic field data

T. Burschil et al.

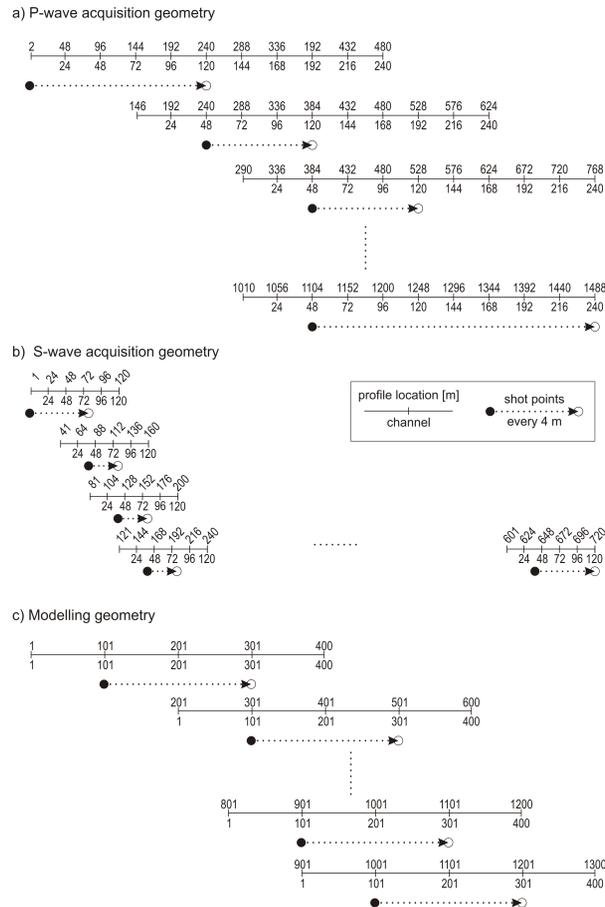


Figure 3. Acquisition geometries for (a) the *P* wave surveys, (b) the *SH* wave surveys, and (c) FD-modelling geometry simplification.

Title Page

Abstract Introduction

Conclusions References

Tables Figures

◀ ▶

◀ ▶

Back Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



FD modelling to
evaluate seismic field
data

T. Burschil et al.

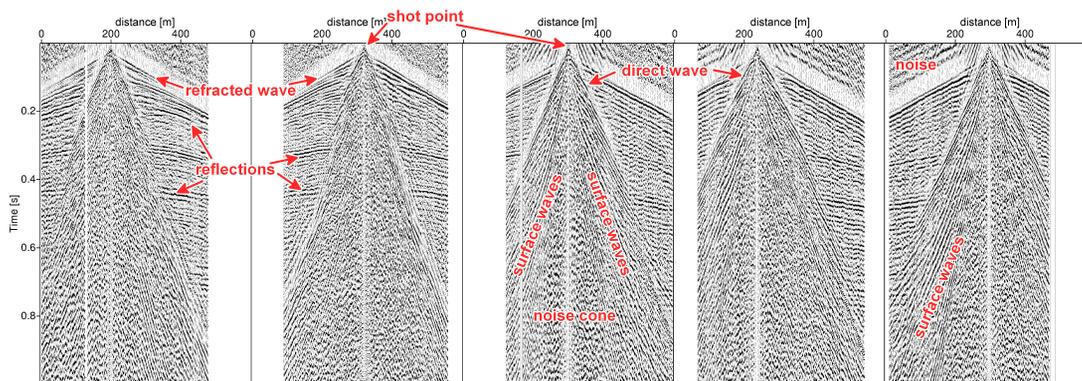


Figure 4. Seismic recordings of five single P wave shots at different locations along the profile. The amplitude is displayed with an automatic gain control (AGC) of 150 ms.

[Title Page](#)[Abstract](#)[Introduction](#)[Conclusions](#)[References](#)[Tables](#)[Figures](#)[⏪](#)[⏩](#)[◀](#)[▶](#)[Back](#)[Close](#)[Full Screen / Esc](#)[Printer-friendly Version](#)[Interactive Discussion](#)

FD modelling to evaluate seismic field data

T. Burschil et al.

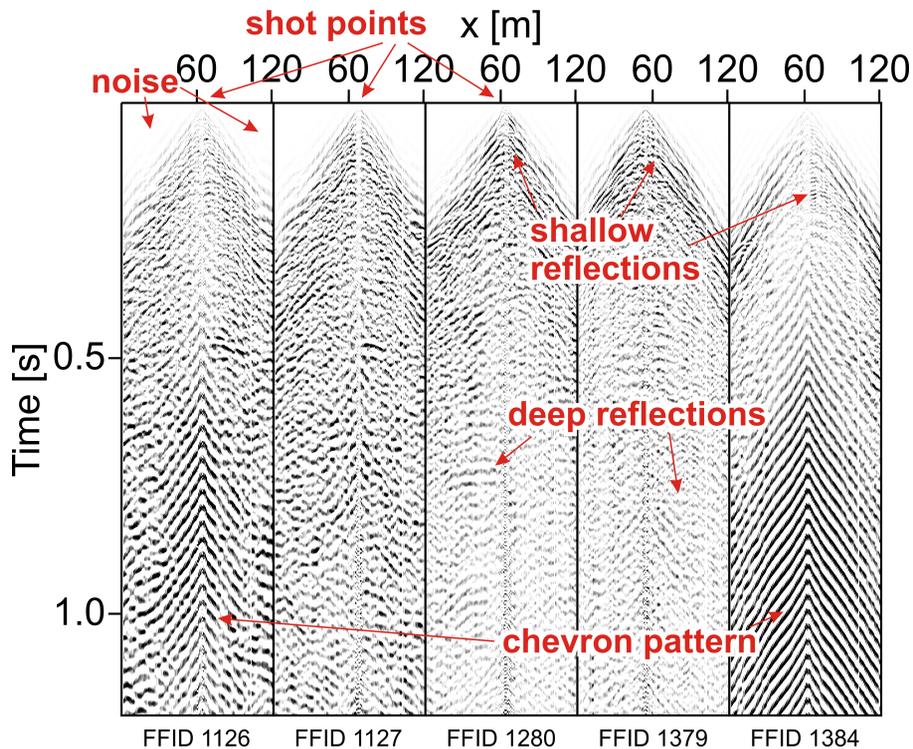


Figure 5. Seismic recordings of five single shear wave shots with spatial divergence correction and AGC of 300 ms applied.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



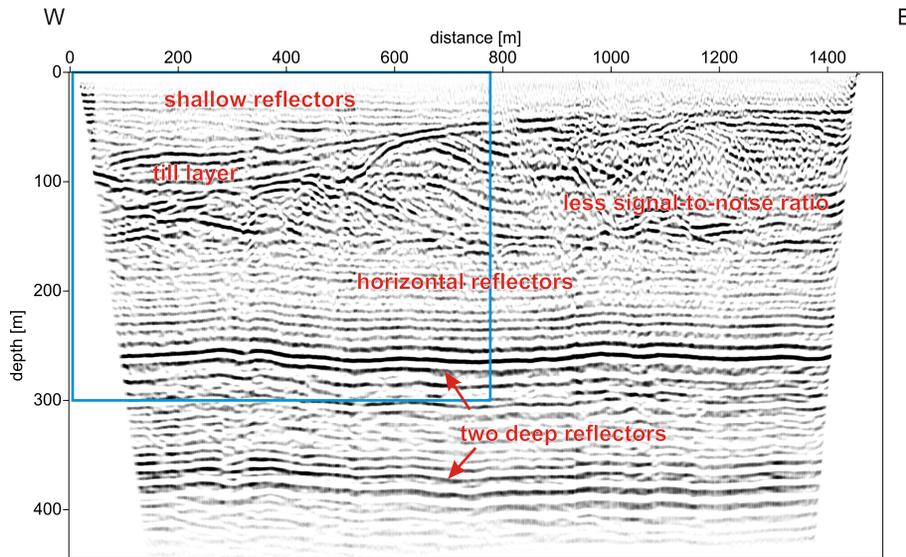


Figure 6. Final time migrated depth section of the *P* wave seismic survey with AGC of 200 ms applied before time-to-depth conversion. The blue box marks the position of the *SH* wave section (Fig. 7).

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page	
Abstract	Introduction
Conclusions	References
Tables	Figures
◀	▶
◀	▶
Back	Close
Full Screen / Esc	
Printer-friendly Version	
Interactive Discussion	



FD modelling to
evaluate seismic field
data

T. Burschil et al.

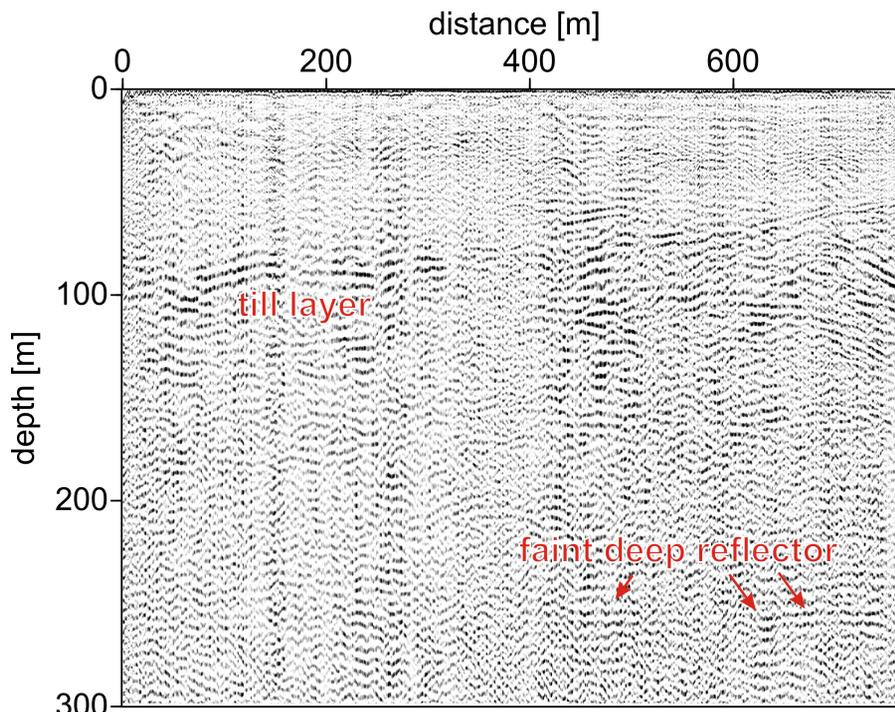


Figure 7. Depth converted stack of the shear wave seismic survey with AGC of 300 m applied after time-to-depth conversion.

[Title Page](#)[Abstract](#)[Introduction](#)[Conclusions](#)[References](#)[Tables](#)[Figures](#)[◀](#)[▶](#)[◀](#)[▶](#)[Back](#)[Close](#)[Full Screen / Esc](#)[Printer-friendly Version](#)[Interactive Discussion](#)

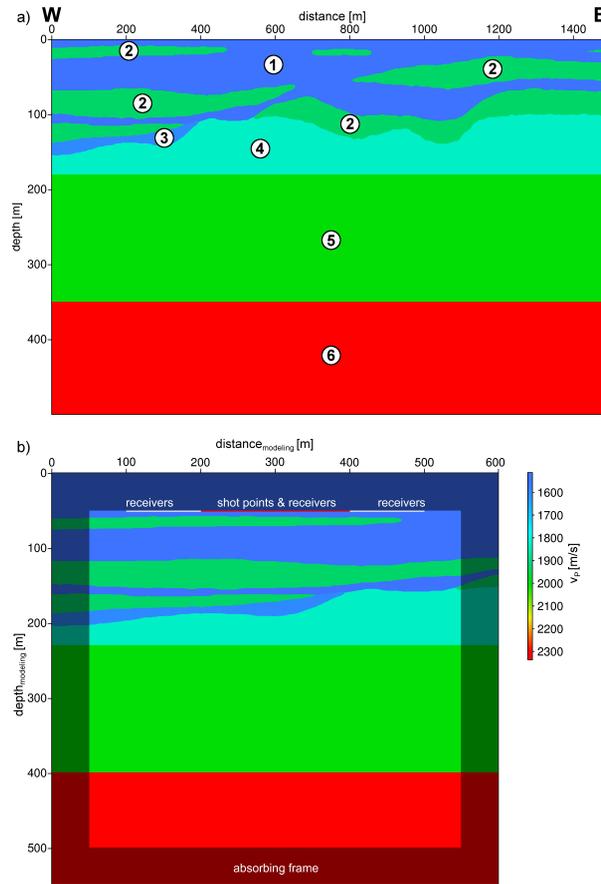


Figure 8. *P* wave velocity model. **(a)** Structural units according to the hydrogeological model, numbers 1–6 mark the units listed in Table 4. **(b)** Westernmost model segment (cf. Sect. 5).

FD modelling to evaluate seismic field data

T. Burschil et al.

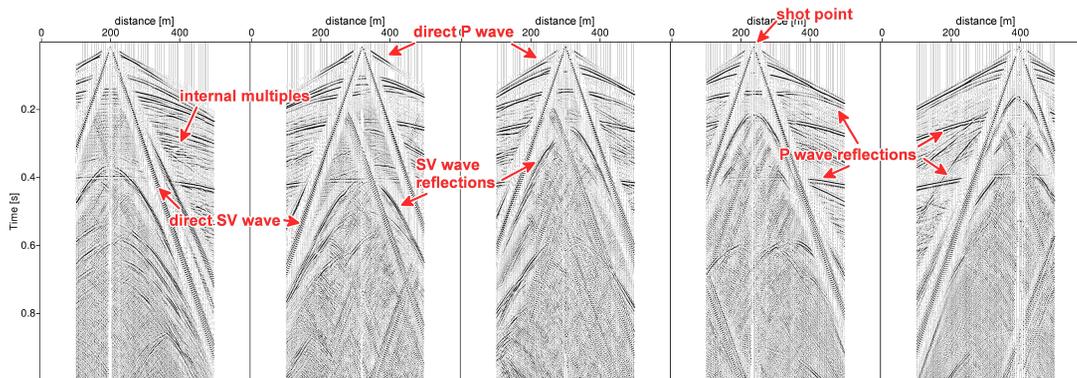


Figure 9. *P* wave shot gathers (as given in Fig. 4), modelled with the hydrogeological model and displayed with 150 ms AGC.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



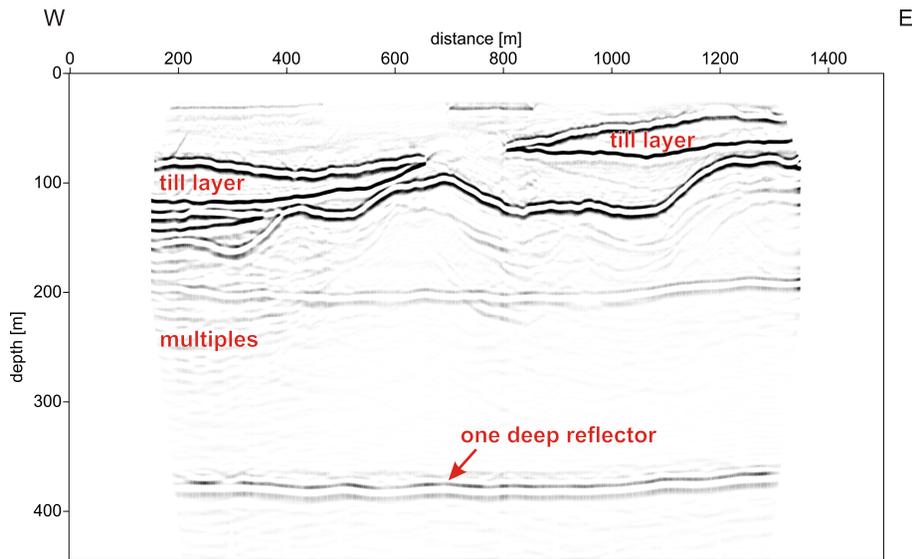


Figure 10. FD-modelled P wave section, based on hydrogeological (Fig. 2) and derived velocity information (Fig. 8). Processed, time-migrated and depth-converted section of 300 single shots with 100 ms AGC.

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page	
Abstract	Introduction
Conclusions	References
Tables	Figures
◀	▶
◀	▶
Back	Close
Full Screen / Esc	
Printer-friendly Version	
Interactive Discussion	



FD modelling to evaluate seismic field data

T. Burschil et al.

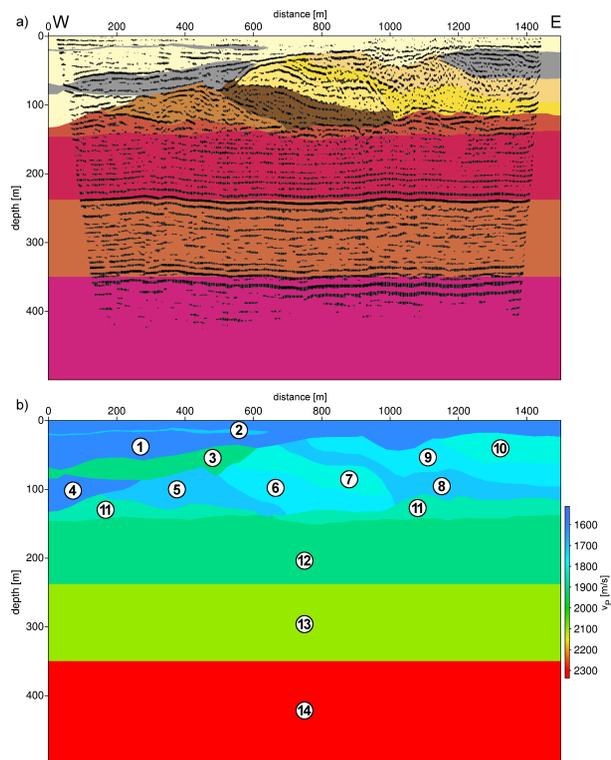


Figure 11. (a) Interpretation of measured field data, used to modify the input model. (b) Modified P wave velocity model. Numbers 1–14 mark the units listed in Table 4.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



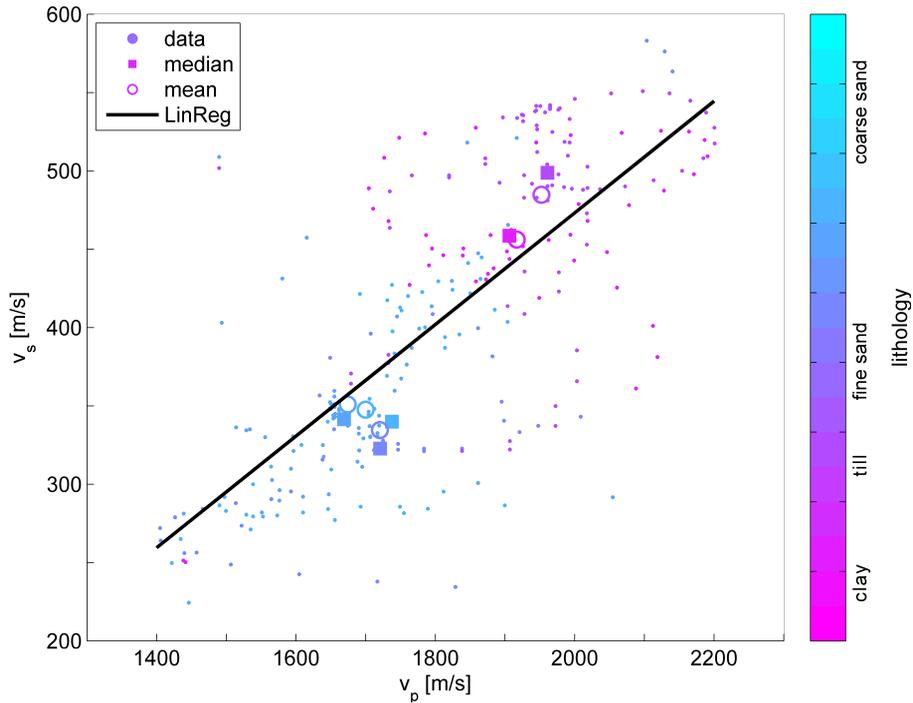


Figure 12. v_P/v_S cross-plot from VSP data colour-coded for different lithologies. Additionally median values, mean values, and the linear regression (LinReg) are indicated. The shear wave velocity was calculated for each P wave velocity with the relation resulting from the linear regression.

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page	
Abstract	Introduction
Conclusions	References
Tables	Figures
◀	▶
◀	▶
Back	Close
Full Screen / Esc	
Printer-friendly Version	
Interactive Discussion	



FD modelling to evaluate seismic field data

T. Burschil et al.

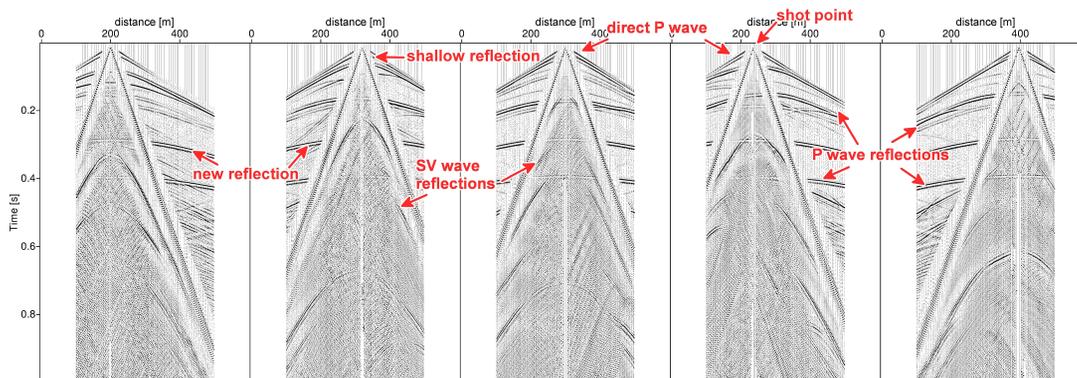


Figure 13. Five different P wave single shots simulated with the modified input model (cf. Fig. 9). Shot gathers are displayed with 150 ms AGC.

Title Page

Abstract

Introduction

Conclusions

References

Tables

Figures



Back

Close

Full Screen / Esc

Printer-friendly Version

Interactive Discussion



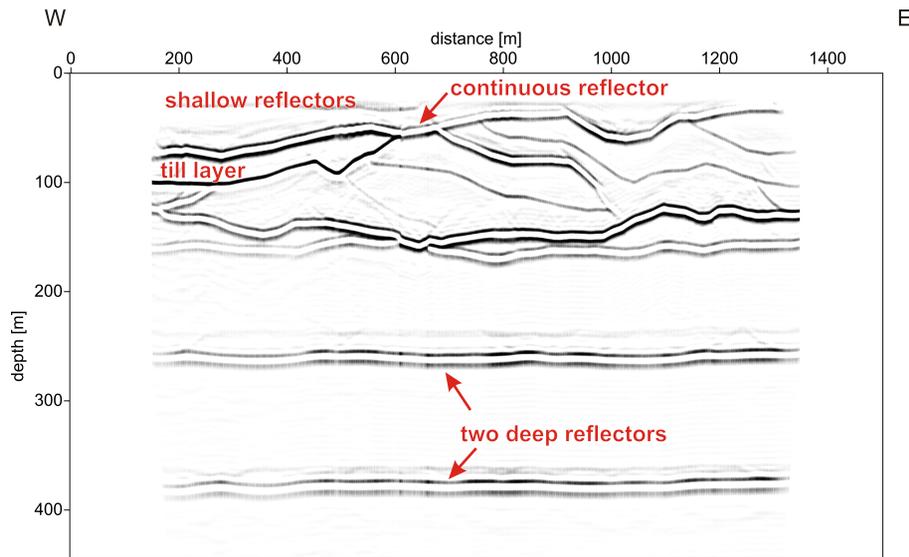


Figure 14. FD-modelling P wave section resulting from the modified input model. Processed, time-migrated and depth-converted section of 300 stacked single shots with AGC of 100 ms (cf. Fig. 10).

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page	
Abstract	Introduction
Conclusions	References
Tables	Figures
◀	▶
◀	▶
Back	Close
Full Screen / Esc	
Printer-friendly Version	
Interactive Discussion	



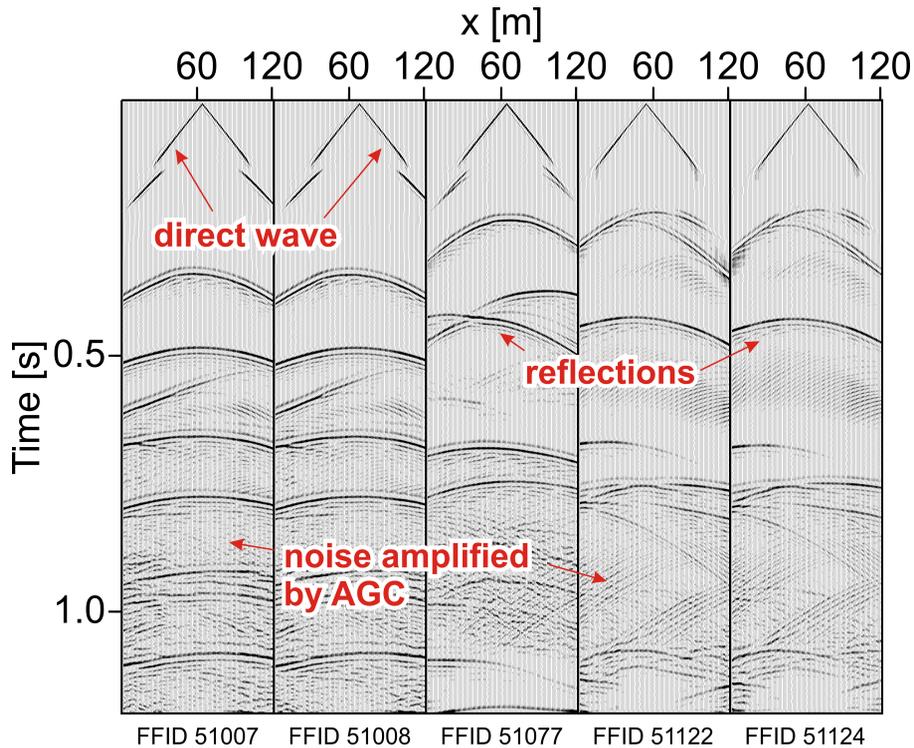


Figure 15. Shear wave FD-modelling shot gathers resulting from the modified input model (cf. Fig. 8). Five shot gathers with 120 m spread, amplified by 300 ms AGC, are displayed.

FD modelling to evaluate seismic field data

T. Burschil et al.

Title Page	
Abstract	Introduction
Conclusions	References
Tables	Figures
◀	▶
◀	▶
Back	Close
Full Screen / Esc	
Printer-friendly Version	
Interactive Discussion	



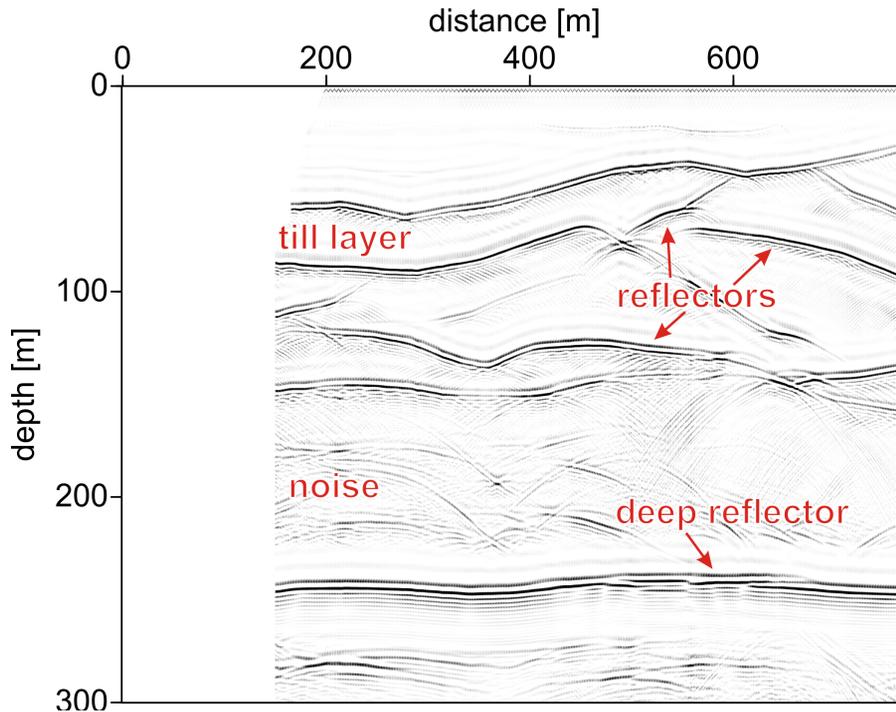


Figure 16. Shear wave FD-modelling section resulting from the modified input model (cf. Fig. 7). Processed, stacked and depth-converted section of 300 stacked single shots with AGC of 300 ms applied.

**FD modelling to
evaluate seismic field
data**

T. Burschil et al.

Title Page	
Abstract	Introduction
Conclusions	References
Tables	Figures
◀	▶
◀	▶
Back	Close
Full Screen / Esc	
Printer-friendly Version	
Interactive Discussion	

